GATE CIVIL ENGINEERING 2006 (CE)

GATE Question Paper Civil Engineering 2006

Q.1-Q.20 Carry One Mark Each

1.	Solutio	n for the system	by the set of	f equations	4y + 3z	= 8 ; 2x □ z-=	2; and 3x	x + 2y = 5 is	
	(a) (c)	x = 0; y = 1; z x = 1; y = 1/2	z = 4/3 ; z = 2	(b) (d)	x = 0; nonexi	y = 1/2; z = 2 stent			
2.	The dif	fferential equation	on $\frac{dy}{dx} = 0.25$	y ² is to be	solved u	sing the backw	ard (impli	cit) Euler's met	thod
	x = 1?								of y at
	(a)	1.33	(b) 1.6	7	(c)	2.00	(d)	2.33	
3.	(a) (b) (c) (d)	no stress shoul tensile stress a shear stress ac no point on it s	d be acting o cting on it mu ting on it mu should be und	n it ust be zero st be zero ler any stre	ess			e is	
4.	Mohr's	circle for the sta	ate of stress o	lefined by	30 0 0 0 30	MPa is a circl	e with		
	(a) (b) (c) (d)	center at (0,0) center at (0,0) center at (30,0) center at (30,0)	and radius 30 and radius 60 and radius 3	0 MPa 0 MPa 30 MPa					
5.	The bu	ıckling load P = I	P _{cr} for the col	umn AB in	the figur	e, as K _⊤ approa	aches infin	ity, become □	$\alpha^{2}_{L^{2}}$,
	where	is equal to	TA NOTE OF THE PROPERTY OF TH		ıral rigidity sional sprii	/ EI ng of stiffness K	г		_
	(a)	0.25	(b) 1.00	<i>//////</i> 0	(c)	2.05	(d)	4.00	
6.		shaft of diamete at its cross-secti			ing mom	ent T at its end	ds. The ma	aximum norma	l stress
	(a)	zero	(b) $\frac{16^{-1}}{\Box dz}$		(c)	32T □d ∄	(d)	64Τ □d ἶ	
7.		characteristic stro test results are e	ength of cond	crete f _{ck} is o	defined a	the strength b		h not more tha	ın 50%

(c)

 f_{m}

(d)

 $f_m + 1.645S$

f_m□1.645S

standard deviation S would be

 $f_m \square 0.1645S$

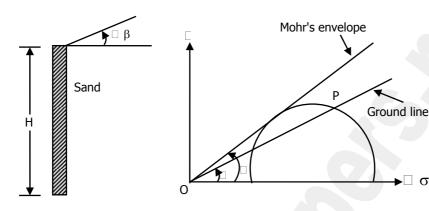
(a)

(b)

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- 8. The range of void ratio between which quick sand conditions occurs in cohesion less granular soil deposits is
 - (a) 0.4□0.5
- (b) 0□6⊟0.7
- (c) 0.8□0.9
- (d) 1.0₁-1
- 9. Figure given below shows a smooth vertical gravity retaining wall with cohesionless soil backfill having an angle of internal friction

 In the graphical representation of Rankine's active earth pressure for the retaining wall shown in figure, length OP represents



- (a) vertical stress at the base
- (b) vertical stress at a height H/3 from the base
- (c) lateral earth pressure at the base
- (d) lateral earth pressure at a height H/3 from the base
- 10. Which of the following statement is NOT true in the context of capillary pressure in soils?
 - (a) Water is under tension in capillary zone
 - (b) Pore water pressure is negative in capillary zone
 - (c) Effective stress increases due to capillary pressure
 - (d) Capillary pressure is more in coarse grained soils
- 11. A channel with a mild slope is followed by a horizontal channel and then by a steep channel. What gradually varied flow profiles will occur?
 - (a) M_1, H_1, S_1
- (b) M_2, H_2, S_2
- (c) M_1, H_2, S_3
- (d) M_1, H_2, S_2
- 12. To provide safety against piping failure, with a factor of safety of 5, what should be he maximum permissible exit gradient for soil with specific gravity of 2.5 and porosity of 0.35?
 - (a) 0.155
- (b) 0.176
- (c) 0.195
- d) 0.213
- 13. Identify the FALSE statement from the following: The specific speed of the pump increases with
 - (a) increase in shaft speed
- (b) increase in discharge
- (c) decrease in gravitational acceleration
- (d) increase in head
- 14. For steady flow to a fully penetrating well in a confined acquifer, the drawdowns at radial distances of r_1 and r_2 from the well have been measured as s_1 and s_2 respectively, for a pumping rate of Q. The transmissivity of the aquifer is equal to
 - (a) $\frac{Q}{2} \frac{\ln \frac{r_2}{r_1}}{(s_1 \pi \Box s_2)}$

- (b) $\frac{Q}{2\Box} \frac{\ln(r_2 \Box r_T)}{(s_1 \Box s_2)}$
- (c) $\frac{Q}{2\Box} \ln \frac{r_2/r_1}{\pi s_2/s_1}$
- (d) $2\Box Q_{\overline{t}} \frac{r_2 \Box r_{\overline{1}}}{\ln \frac{s_2}{s_4}}$

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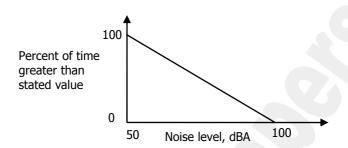
15. To determine the BOD_5 of a wastewater sample, 5, 10 and 50 mL aliquots of the wastewater were diluted to 300 mL and incubated at 20 \square C°in BOD bottles for 5 days.

Sl. No.	Wastewater Volume,	Initial DO, mg/L	DO After 5 days, mg/L					
1.	5	9.2	6.9					
2.	10	9.1	4.4					
3.	50	8.4	0.0					

Based on the data, the average BOD₅ of the wastewater is equal to

- (a) 139.5 mg/L
- (b) 126.5 mg/L
- (c) 109.8 mg/L
- (d) 72.2 mg/L

16. The cumulative noise power distribution curve at a certain location is given below.



The value of L₄₀ is equal to

- (a) 90 dBA
- (b) 80 dBA
- (c) 70 dBA
- (d) 60 dBA

17. A synthetic sample of water is prepared by adding 100 mg Kaolinite (a clay minerla), 200 mg glucose, 168 mg NacI, 120 mg MgSO₄, and 111 mg $CaCI_2$ to 1 liter of pure water. The concentrations of total solids (TS) and fixed dissolved solids (FDS) respectively in the solution in mg/L are equal to

- (a)
- 699 and 599
- 599 and 399
- (c) 699 and 199
- (d) 699 and 399

18. If aggregate size of 50□40 mm is to be tested for finding out the portion of elongated aggregates using length gauge, the slot length of the gauge should be

- (a) 81 mm
- (b) 45 mm
- (c) 53 mm
- (d) 90 mm

19. Name the traffic survey data which is plotted by means of "Desire lines".

(a) Accident

- (b) Classified volume
- (c) Origin and Destination
- (d) Speed and Delay

20. In case of governing equations for calculating wheel load stresses using Wesergaard's approach, the following statements are made.

- I. Load stress are inversely proportional to wheel load
- II. Modulus of subgrade reaction is useful for load stress calculation
- (a) Both statements are TRUE
- (b) I is TRUE and II is FALSE
- (c) Both statements are FALSE
- (d) I is FALSE AND II is TRUE

Q. 21 to Q.75 carry two marks each

21. For a given matrix $A = \begin{bmatrix} 2 & 2 & 3 \\ 2 & +1 & 6 \\ 1 & 2 & 0 \end{bmatrix}$ one of the eigenvalues is 3. The other two eigenvalues are

- (a) 2, □5–
- (b) 3, □5–
- (c) 2, 5
- (d) 3, 5

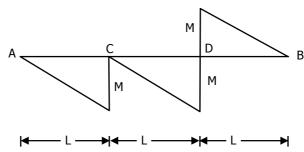
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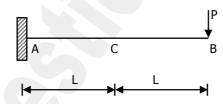
22.	The directional derivative of $f(x,y,z) = 2x^2+3y^2+z^2$ at the point P: (2, 1, 3) in the direction of the vector $a = i - 2k$ is											
	(a)	□2-785	(b)	□2-145	(c)	- 1.789	(d)	1.000				
23.	studen mean of entire of of the	ts. It is found to of 6.6 and stand class are 5.5 and students of all l	hat the sed dard deviated and 4.2, reso patches to	essional marks of ation of 2.3. The spectively. It is to have the same	f studen mean a lecided mean a	its in Engineering and standard dev by the course ins	g Drawin viation of structor friation as	each consisting of 30 g in batch C have a the marks for the to normalize the marks that of the entire				
	(a)	6.0	(b)	7.0	(c)	8.0	(d)	9.0				
24.	A 2 nd d	egree polynom	ial, f(x), h	nas values of 1,4	, and 15	5 at $x = 0$, and 2	, respec	tively. The integral				
							<i>(</i> 1)	2				
	(a)	$\frac{1}{3}$	(b)	$\Box \frac{2}{3}$	(c)	0	(d)	2 3				
25.	What is	s the area com	mon to th	e circles r = 🗆 a	nd r = 2	la cos □?θ						
	(a)	$0.524 a^2$	(b)	$0.614 a^2$	(c)	0.147 a ²	(d)	1.228 a ²				
26.					he integ	ral (integration b	eing tak	en in counter				
	CIOCKW	ise direction) [$3z \square i -$	ZIS								
	(a)	2□ π 81 ^π 4□i-π	(b)	<u>□</u> π6⊟i π	(c)	<u>4□</u> π6⊡i–π	(d)	1				
27.	picked	for inspecion ((i.e., each	has the same o	chance o			tors are randomly s the probability that				
	(a)	1/2	(b)	$\frac{1}{3}$	(c)	$\frac{1}{4}$	(d)	1 5				
28.	instant	aneous surface	area due	to evaporation.	If the i		the ball	proportional to is is 2 cm and the				
	(a)	6 months	(b)	9 months	(c)	12 months	(d)	infinite time				
29.	The so	lution of the dif	ferential	equation								
25.				•	. — O is							
	$\frac{x}{dx}$	+ 2xy ⊔ X+1	= u giver	n that at $x = 1$,	y = U IS							
	(a)	$\frac{1}{2}$ $\frac{1}{x}$ $\frac{1}{2x^2}$	(b)	$\frac{1}{2}$ $\frac{1}{x} = \frac{1}{2x^2}$	(c)	$\frac{1}{2}$ $\frac{1}{x} + \frac{1}{2x^2}$	(d)	$\Box \frac{1}{2} \stackrel{1}{} \Box \frac{1}{2x^2}$				

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30. A simply supported beam AB has the bending moment diagram as shown in the following figure. The beam is possibly under the action of following loads:

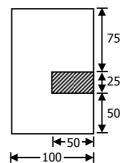


- (a) Couples of M at C and 2M at D
- (b) Couples of 2M at C and M at D
- (c) Concentrated loads of M/L at C and 2M/Lat D
- (d) Concentrated load of M/L at C and couple of 2M at D.
- 31. For the section shown below, second moment of the area about an axis d/4 distance above the bottom of the area is
 - (a) $\frac{bd^3}{48}$
- $(b) \qquad \frac{bd^3}{12}$
- (c) $\frac{7bd^3}{48}$
- (d) $\frac{bd^3}{3}$
- 32. Consider the beam AB shown in the figure below. Part AC of the beam is rigid while Part CB has the flexural rigidity EI. Identify the correct combination of deflection at end B and bending moment.



- (a) $\frac{PL^3}{3FI}$, 2Pl
- (b) $\frac{PL^3}{3EI}$, P
- (c) $\frac{8PL^3}{2EL}$, 2PI
- (d) $\frac{8PL^3}{3EI}$, PL

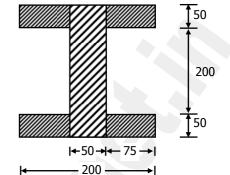
33. A beam with the cross-section given below is subjected to a positive bending moment (causing compression at the top) of 16 kN m acting around the horizontal axis. The tensile force acting on the hatched area of the cross-section is



- (a) zero
- (b) 5.9 kN
- (c) 8.9 kN
- (d) 17.8 kN

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34. T-section of a beam is formed by gluing wooden planks as shown in the figure below. If this beam transmits a constant vertical shear force of 3000 K, the glue at any of the four joints will be subjected to a shear force (in kN per meter length) of



35. If a beam of rectangular cross-section is subjected to a vertical shear force V, the shear force carried by the upper one-third of the cross-section is

(b)
$$\frac{7V}{27}$$

(c)
$$\frac{8}{2}$$

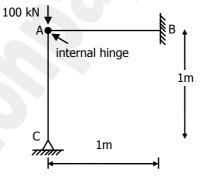
(d)
$$\frac{V}{3}$$

36. A thin-walled long cylindrical tank of inside radius r is subjected simultaneously to internal gas pressure p and axial compressive force F at its ends. In order to produce 'pure shear' state of stress in the wall of the cylinder, F should be equal to

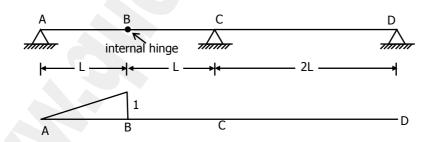
37. Vertical reaction developed at B in the frame be-low due to the applied load of 100 kN (with 150, 000mm² crosssectional area and

 $3.125 \square 10^9 \text{ mm}^4 \text{ moment of inertia for both members) is}$





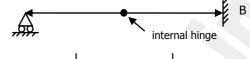
38. Consider the beam ABCD and the influence line as shown below. The inflience the pertains to



- (a) reaction at A, R_A
- (b) shear force at B, V_B
- (c) shear force on the left of C, V_C^{\Box}
- (d) shear force on the right of C, V_C^{-+}

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- 39. Carry-over factor C_{AB} for the beam shown in the figure below is
 - (a) $\frac{1}{4}$
- (b) $\frac{1}{2}$



- (c) $\frac{3}{4}$
- (d) 1
- 40. Assuming concrete below the neutral axis to be cracked, the shear stress across the depth of a singly-reinforce rectangular beam section
 - (a) increases parabolically to the neutral axis and then drops suddently to zero value.
 - (b) increases parabolically to the neutral axis and then remains constant over the remaining depth
 - (c) increases linearly to the neutral axis and then remains constant up to the tension steel
 - (d) increases parabolically to the neutral axis and then remains constant up to the tension steel.
- 41. As per IS: 456-2000, consider the following statements
 - I. The modular ratio considered in the working stress method depends on the type of steel used
 - II. There is an upper limit on the nominal shear stress in beams (even withshear reinforcement) due to the possibility of crushing of concrete in diagonal compression.
 - III. A rectangular slab whose length is equal to its width may not be a two-way slab for some support conditions.

The TRUE statements are

- (a) only I and II
- (b) only II and III (c)
- only I and III
- , II and III
- 42. In the design of welded tension members, consider the following statements:
 - I. The entire cross-sectional area of the connected leg is assumed to contribute to the effective area in case of angles.
 - II. Two angles back-to-back and tack-welded as per the codal requirements may be assumed to behave as a tee section.
 - III. A check on slenderness ratio may be necessary in some cases.

The TRUE statements are

- (a) only I and II
- (b) only II and III (c)
- only I and III
- (d) I, II and III

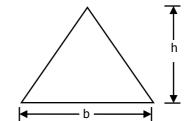
- 43. Consider the following statements
 - I. Effective length of a battened column is usually increased to account for the additional load on battens due to the lateral expansion of columns.
 - II. As per IS: 800-1984, permissible stress in bending compression depends on both Euler buckling stress and the yield stress of steel.
 - III. As per IS: 800-1984, the effective length of a column effectively held in position at both ends but not restrained against rotation, is taken to be greater than that in the ideal end conditions.

The TRUE statements are

(a) only I and II (b) only II and III (c) only I and III (d) I, II and III

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44. When the triangular section of a beam as shown below becomes a plastic hinge, the compressive force acting on the section (with y denoting the yileld stress) becomes



(a)
$$\frac{bh\Box_{y^t}}{4}$$

$$(b) \qquad \frac{2bh_y}{9}$$

(c)
$$\frac{bh\Box_{y^{c}}}{2}$$

(d)
$$\frac{bh\Box_{y^c}}{3}$$

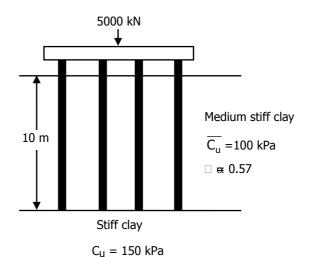
- 45. Consider the following statements:
 - I. The width-thickness ratio limitations on the plate elements under compression in steel members are imposed by IS: 800-1984 in order to avoid fabrication difficulties.
 - II. In a doubly reinforced concrete beam, the strain in compressive reinforcement is higher than the strain in the adjoining concrete.
 - III. If a cantilever I-section supports slab construction all along its length with sufficient friction between them, the permissible bending stress in compression will be the same as that in tension.

The TRUE statements are

- (a) only I and II
- (b) only II and III
- (c) only I and III
- (d) I, II and III
- 46. List I below gives the possible types of failure for a finite soil slope and List II gives the reasons for these different types of failure. Match the items in List I with the items in List II.

List - I

- P Base failure
- Q Face Failure
- R Toe failure
- (a) P-1 Q-2 R-3
- (c) P-2 Q-1 R-3
- List-II
- 1. Soils above and below the toe have same strength
- 2. Soil above the toe is comparatively weaker
- 3. Soil above the toe is comparatively stronger
 - (b) P-2 Q-3 R-1
 - (d) P-3 Q-2 R-1
- 47. For the soil profile shown in figure below, the minimum number of precast concrete piles of 300 mm diameter required to safety carry the load for a given factor of safety of 2.5 (assuming 100% efficiency for the pile group) is equal to



- (a) 10
- (b) 15
- (c) 20
- (d) 25

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48.	compaction. A soil sample weighing 23 g was taken from the mould and overdried for 24 hours at a temperature of 110° C. Weight of the dry sample was found to be 20 g. Specific gravity of soil solids G = 2.7. The theoretical maximum value of the dry unit weight of the soil at that water content is equal to												
	(a)	4.67 kN/m ³	(b)	11.5 kN/m ³	(c)	16.26 kN/m ³	(d)	8.85 kN/m ³					
49.	A sample of saturated cohesionless soil tested in a drained triaxial compression test should of internal friction of $30\Box$ The deviatoric stress at failure for the sample at a confining part kPa is equal to												
	(a)	2000 kPa	(b)	400 kPa	(c)	600 kPa	(d)	800 kPa					
50.	The thickness of the laminar boundary layer on a flat plate at a point A is 2 cm and at a point B, 3 downstream of A, is 3 cm. What is the distance of A from the leading edge of the plate?												
	(a)	0.50 m	(b)	0.80 m	(c)	1.00 m	(d)	1.25 m					
51.	The velocity field for flow is given by $v (5x 6y 7z)\hat{i} (6x = \boxed{5} = +9z)\hat{j} (3x + \boxed{2} = +2z)\hat{k}$ and the density varies as $\boxed{9} \boxed{9} \cancel{6} = 2z$ and the density varies as $\boxed{9} \boxed{9} \cancel{6} = 2z$ and the density varies as $\boxed{9} \boxed{9} \cancel{6} = 2z$												
	(a)	-12	(b)	-10	(c)	-8	(d)	10					
52.	root zo the irrig	ne depth is 1.0 r gation efficiency	m, the pe is 60%,	ermanent wilting	point is the freq	10% and the country of irrigation	nsumpti	a particular crop, the ve use is 15 mm/d. If hat the moisture					
	(a)	5d	(b)	6d	(b)	9d	(d)	15 d					
53.						tionless channel nergy loss is 1 n		ould be the pre-jump					
	(a)	0.2	(b)	0.3 m	(c)	0.8 m	(d)	0.9 m					
54.	negligil mm/hr	ole. The rainfall verspectively and	was idea I the infil	lized as 3 one ho	our storr lized as	ostractions other ns of intensity 10 a Horton curve,	0 mm/hr	, 20 mm/hr and 10					
	(a)	10.00 mm	(b)	11.33 mm	(c)	12.43 mm	(d)	13.63 mm					
55.	design using S used th	is based on the strickler's equation	Manning on and re	's equation with esults in a norma	the rough	of 1.0 m. By mis	it obtain take, ho	eter width. The ed from the grain size wever the engineer at should be the					
	(a)	0.32 m	(b)	0.50 m	(c)	2.00 m	(d)	3.20 m					
56.	laborat importa	ory flume using	water (v be the l	=10 ⁻⁶ m ² /s) as th ength scale (i.e.	e flowin	² /s) in an open c g fluid. If both g prototype to mo	ravity ar						
	(a)	1	(b)	22	(c)	63	(d)	500					

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57.	The mean indoor airborne chlo	: T = 293 K, P	= 1 atmosphere, R	$a = 82.05 \square 10^{-6} \text{ atm.m}^3 / \text{ mol}^{-6}$	-K,
	Atomic weights : $C = 12$, $H=1$, CI= 35.5. This	concentration exp	ressed in parts per billion (vo	olume
	basis, ppbv) is equal to				
	(-) 1 00 ·····l···	/I- \	0.20		

(a) 1.00 ppbv (b) 0.20 ppbv (c) 0.10 ppbv (d) 0.08 ppbv

58. The composition of a certain MSW sample and specific weights of its various components are given below.

Component	Percent by Weight	Specific Weight (kg/m³)
Food waste	50	300
Dirt and Ash	30	500
Plastics	10	65
Wood and Yard waste	10	125

Specific weight (kg/m³) of the MSW sample is

(a) 319 (b) 217 (c) 209 (d) 199

59. A subgrade soil sample was tested using standard CBR apparatus and the observations are given below.

Load, kgPenetration, mm60.52.580.55.0

Assuming that the load-penetration curve is convex throughout, the CBR value (%) of the sample is

(a) 6.5

(b) 5.5

(c) 4.

(d) 3.9

60. A vehicle moving at 60 kmph on an ascending gradient of a highway has to come to stop position to avoid collision with a stationary object. The ratio of lag to brake distance is 6:5. Considering total reaction time of the driver as 2.5 Considering total reaction time of the driver as 2.5 seconds and the coefficient of longitudinal friction as 0.36, the value of ascending gradient (%) is

(a) 3.3

(b) 4.8

(c) 5

(d) 6.8

61. For designing a 2-phase fixed type signal at an intersection having North-South and East-West road where only straight ahead traffic permitted, the following data is available.

Parameter Design Hour	North	South	East	West
Flow (PCU/hr)	1000	700	900	550
Saturation Flow (PCU/hr)	2500	2500	3000	3000

Total time lost per cycle is 12 seconds. The cycle length (seconds) as per Webster's approach is

(a) 67

(b)

7

(c) 87

(d)

91

62. On an urban road, the free mean speed was measured as 70 kmph and the average spacing between the vehicles under jam condition as 7.0 m. The speed-flow-density equation is given by

where U = space-mean speed (kmph); U_{sf} = free mean speed (kmph); k = density (veh/km); k = jam density (veh/km); q = flow (veh/hr). The maximum flow (veh/hr) per lane for this condition is equal to

(a) 2000

(b) 2500

(c)

3000

(d) None of these

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- 63. At a horizontal curve portion of a 4 lane undivided carriageway, a transition curve is to be introduced to attain required superelevation. The design speed is 60 kmph and radius of the curve is 245 m. Assume length of wheel base of a longest vehicle as 6 m, superelevation rate as 5% and rate of introduction of this superelevation as 1 in 150. The length of the transition curve (m) required, if the pavement is rotated about inner edge is.
 - 81.4 (a)
- (b) 85.0
- 91.5 (c)
- 110.2 (d)
- 64. Using IRC: 37 – 1984 "Guidelines for the Design of Flexible Pavements" and the following data, choose the total thickness of the pavement.

No. of commercial vehicles when construction is completed = 2723 veh/day

Annual growth rate of the traffic = 5.0%

Design life of the pavement = 10 years

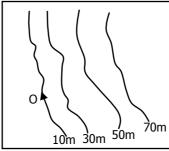
Vehicle damage factor = 2.4

CBR value of the subgrade soil = 5%

Data for 5% CBR value

No. o	f Standard Axe	ls, msa	Total Thickes,	, mm	
20			620		
25			640		
30			670		
40			700		
(a)	620 mm	(b)	640 mm	(c)	670 mm

- (a) 620 mm
- (b) 640 mm
- 670 mm
- (d) 700 mm
- 65. The observed magnetic bearing of a line OE was found to be 185□ It was later discovered that station O had a local attraction of + 1.5□ The true bearing of the line OE, considering a magnetic a magnetic declination of 3.5 ₱ shall be
 - 180□ ° (a)
- (b) 187□ °
- (c) 190□ °
- (d) 193□ °
- 66. A Bench Mark (BM) with Reduced Level (RL) = 155.305 m has been established at the floor of a room. It is required to find out the RI of the underside of the roof (R) of the room using Spirit Leveling. The Back Sight (BS) to the BM has been observed as 1.500 m whereas the ForeSight (FS) to R has been observed as 0.575 m (Staff held inverted). The RL (m) of R will be
 - 155.880 (a)
- 156.230 (b)
- (c) 157,380
- 157.860 (d)
- 67. Consider the following figure, which is an extract from a contour map (scale = 1:20,000) of an area. An alignment of a road at a ruling gradient of 4% is to be fixed from the point O and beyond. What should be the radius of the arc with O as the center to get the point of a alignment on the next contour on the map.



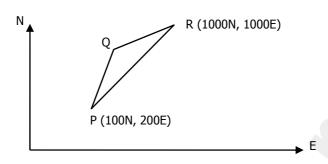
0.025 cm

(b) 0.25 cm

(c) 2.5 cm (d) 5.0 cm

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68. In the figure given below, lengths PQ(WCB:30°) and QR (WCB:45°) respectively up to three places of decimal are



- (a) 273.205, 938.186
- (b) 273.205, 551.815
- (c) 551.815, 551.815
- (d) 551.815, 938.186
- 69. During a leveling work along a falling gradient using a Dumpy level and a Staff of 3m length, following successive readings were taken: 1.785, 2.935, 0.360, 1.320. What will be the correct order of booking these four readings in a level book? (BS: Back Sight, IS: Intermediate Sight, FS: Fore Sight)
 - (a) BS, FS, BS, FS
- (b) BS, IS, FS, FS
- (c) BS, IS, IS, FS
- (d) BS, IS, BS, FS

Common Data Questions Common Data for Question 70, 71:

Laboratory sieve analysis was carried out on a soil sample using a complete set of standard IS sieves. Out of 500g of soil used in the test, 200g was retained on IS 600 sieve, 250g was retained on IS 500 sieve and the remaining 50g was retained on IS 425 \$\sigma\$ sieve.

- 70. The coefficient of uniformity of the soil is
 - (a) 0.9
- (b) 1.0
- (c) 1.1
- (d) 1.2

- 71. The classification of the soil is
 - (a) SP
- (b) SW
- (c) GP
- (d) GW

Common Data for Questions 72,73:

For a catchment, the S-curve (or S-hydrograph) due to a rainfall of intensity 1cm/hr is given by $Q = 1 - (1+t) \exp(-1)$ (t in hr and Q in m³/s).

- 72. What is the area of the catchment?
 - (a) 0.01 km^2
- (b) 0.36 km^2
- (c) 1.00 km^2
- (d) 1.28 km²
- 73. What will be the ordinate of a 2-hour unit hydrograph for this catchment at t=3 hour?
 - (a) $0.13 \text{ m}^3/\text{s}$
- (b) $0.20 \text{ m}^3/\text{s}$
- (c) $0.27 \text{ m}^3/\text{s}$
- (d) $0.54 \text{ m}^3/\text{s}$

Common Data for Questions 74,75:

In a rapid sand filter, the time for reaching particle breakthrough (T_B) is defined as the time elapsed from start of filter run to the time at which the turbidity of the effluent from the filter is greater than 2.5 NTU. The time for reaching terminal head loss (T_H) is defined as the time elapsed from the start of the filter run to the time when head loss across the filter is greater than 3m.

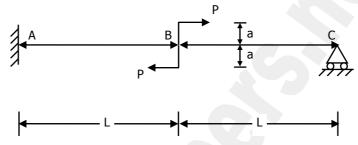
- 74. The effect of increasing the filter depth (while keeping all other conditions same) on T_B and T_H is
 - (a) T_B increases and T_H decreases
- (b) both T_B and T_H increase
- (c) T_B decreases and T_H increases
- (d) both T_B and T_H decreases

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- 75. The effect of increasing the filter loading rate (while keeping all other conditions same) on T_B and T_H is
 - (a) T_B increases and T_H decreases
- (b) both T_B and T_H increases
- (c) T_B decreases and T_H increases
- (d) both T_B and T_H decreases

<u>Linked Answer questions : Q.76 to Q.85 Carry two marks each</u> <u>Statement of Linked Answer Question 76 and 77</u>

Consider a propped cantilever beam ABC under two loads of magnitude P each as shown in the figure below. Flexural rigidity of the beam is EI.



- 76. The reaction at C is
 - (a) $\frac{9Pa}{16L}$ (upwards)

(b) $\frac{9Pa}{16l}$ (downwards)

(c) $\frac{9Pa}{8l}$ (upwards)

(d) $\frac{9Pa}{8l}$ (downwards)

- 77. The rotation at B is
 - (a) $\frac{5PLa}{16EI}$ (clockwise)

(b) $\frac{5PLa}{16FI}$ (anticlockwise)

(c) $\frac{59PLa}{16EI}$ (clockwise)

(d) $\frac{59\text{PLa}}{16\text{EI}}$ (anticlockwise)

Statement for Linked Answer Questions 78 and 79:

In the design of beams for the limit state of colapse in flexure as per IS: 456-2000, let the maximum strain in concrete be limited to 0.0025 (in place of 0.0035). For this situation, consider a rectangular beam section with breadth as 250 mm, effective depth as 350 mm, area of tension steel as 1500 mm², and characteristics strengths of concrete and steel as 30Mpa and 250 MPa respectively.

- 78. The depth of neutral axis for the balanced failure is
 - (a) 140 mm
- (b) 156 mm
- (c) 168 mm
- (d) 185 mm
- 79. At the limiting state of collapse in flexure, the force acting on the compression zone of the section is
 - (a) 326 kN
- (b) 389 kN
- (c) 424 kN
- (d) 542 kN

Statement for Linked Answer Questions 80 and 81

The average effective overburden pressure on 10 m thick homogeneous saturated clay layer is 150 kPa. Consolidation test on undisturbed soil sample taken from the clay layer showed that the void ratio decreased from 0.6 to 0.5 by increasing the stress intensity from 100 kPa to 300 kPa. (G=2.65)

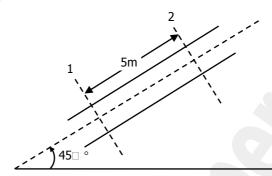
- 80. The initial void ratio of the clay layer is
 - (a) 0.209
- (b) 0.563
- (c) 0.746
- (d) 1.000

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- 81. The total consolidation settlement of the clay layer due to the construction of a structure imposing an additional stress intensity of 200 kPa is
 - (a) 0.10 m
- (b) 0.25 m
- (c) 0.35 m
- (d) 0.50 m

Statement for Linked Answer Questions 82 and 83

An upward flow of oil (mass density 800 kg/m^3 , dynamic viscosity 0.8 kg/m-s) takes place under laminar conditions in an inclined pipe of 0.1 m diameter as shown in the figure. The pressures at section 1 and 2 are measured p1=435 kN/m² and p²= 200 kN/m^2



- 82. The discharge in the pipe is equal to
 - (a) $0.100 \text{ m}^3/\text{s}$
- (b) $0.127 \text{ m}^3/\text{s}$
- (c) $0.144 \text{ m}^3/\text{s}$
- (d) $0.161 \text{ m}^3/\text{s}$
- 83. If the flow is reversed, keeping the same discharge, and the pressure at section 1 is maintained as 435 kN/m^2 , the pressure at section 2 is equal to
 - (a) 488 kN/m^2
- (b) 549 kN/m²
- (c) 586 kN/m²
- (d) 614 kN/m^2

Statement for Linked Answer Questions 84 and 85

A water sample contains the following dissolved

ions. $[Na^+]$ = 56 mg/L; $[Ca^{2+}]$

 $= 40 \text{ mg/L;Mg}^{2+}$

 $= 30 \text{mg/L}; [A1^{3+}]$

= 3mg/L; [HCO₃]

= 190 mg/L; $[Cl^{\square}]$ = 165 mg/L; Water Ph is 7

Atomic weights: Ca:40; Mg: 24; AI:27; H:1, C:12; O:16; Na:23; CI:35.5

- 84. The total hardness of the sample in mg/L as CaCO₃ is
 - (a) 484
- (b) 450
- (c) 242
- (d) 225
- 85. The non-arbonate hardness of the sample in mg/L as $CaCO_3$ is
 - (a) 225
- (b) 156
- (c) 86
- (d) 0

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1	(d)	2	(c)	3	(a)	4	(d)	5	(d)	6	(b)	7	(c)	8	(c)	9	(a)	10	(d)
11	(d)	12	(c)	13	(d)	14	(a)	15	(c)	16	(b)	17	(d)	18	(a)	19	(c)	20	(d)
21	(b)	22	(c)	23	(d)	24	(a)	25	(d)	26	(a)	27	(b)	28	(a)	29	(a)	30	(a)
31	(c)	32	(a)	33	(c)	34	(b)	35	(b)	36	(c)	37	(a)	38	(b)	39	(d)	40	(d)
41	(b)	42	(d)	43	(a)	44	(a)	45	(a)	46	(d)	47	(c)	48	(c)	49	(b)	50	(b)
51	(b)	52	(c)	53	(b)	54	(d)	55	(d)	56	(c)	57	(d)	58	(b)	59	(c)	60	(b)
61	(b)	62	(b)	63	(d)	64	(c)	65	(b)	66	(c)	67	(c)	68	(a)	69	(a)	70	(d)
71	(a)	72	(b)	73	*	74	(a)	75	(d)	76	(c)	77	(a)	78	(b)	79	(b)	80	(b)
81	(d)	82	(b)	83	(d)	84	(c)	85	(c)										